## Forest Hydrology: Lect. 10

## Contents

- The flood hydrograph
- SCS - Curve Number method
- Application of the SCS - CN method at hourly (sub-event) time steps


## The flood and its hydrograph

## Terminology

Streamflow (runoff) = storm runoff + baseflow
or
quickflow + delayed flow

Flood runoff coefficient= (event runoff)/(event rainfall)


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## Prediction of watershed runoff (i.e.: computing the flood event runoff coefficient)

What do we want to know? Total volume of storm runoff given the rainfall

- Soil Conservation Service Method (Curve Number)
- Very widely used in prediction software
- Accounts for effects of soil properties, land cover, and antecedent soil moisture
- Prediction of storm flow depends on total rainfall rather than intensity
- Based on a very simple conceptual model, as follows.


## Prediction of storm runoff volume ('SCS' method)

All quantities expressed in mm of water

Total precipitation, $P$, is partitioned into:

- An initial abstraction, $\boldsymbol{I}_{a}$, the amount of storage that must be satisfied before any flow can begin. This is poorly defined in terms of process, but is roughly equivalent to interception and infiltration that occurs before runoff.
- Thus, $\left[P-I_{a}\right]$ is the 'excess precipitation' (after the initial abstraction) or the 'potential runoff'.
- Retention, $F$, the amount of rain falling after the initial abstraction is satisfied, but which does not contribute to the storm flow. This is equivalent to volume of water that is infiltrated.
- Storm runoff $R_{s}$

It is assumed that a watershed can hold a certain maximum amount of precipitation, $S_{\text {max }}$

$$
\begin{equation*}
S_{\max }=I_{a}+F_{\infty} \tag{1}
\end{equation*}
$$

where $F_{\infty}$ is the total amount of water retained as $t$ becomes very large (i.e. in a long, large storm).
It is the cumulative amount of infiltration

It is also assumed that during the storm (and particularly at the end of the storm)

$$
\begin{equation*}
\frac{R_{s}}{\left[P-I_{a}\right]}=\frac{F}{S_{\max }} \tag{2}
\end{equation*}
$$

- The idea is that "the more of the potential storage that has been exhausted (cumulative infiltration, $F$, converges on $S_{\text {max }}$ ), the more of the 'excess rainfall', or 'potential runoff', $P-I_{a}$, will be converted to storm runoff."
- The scaling is assumed to be linear.
- One more relationship that is known by definition (balance):

$$
\begin{equation*}
F=P-I_{a}-R_{s} \tag{3}
\end{equation*}
$$

- Combination of these leads to

$$
\begin{equation*}
R_{s}=\frac{\left[P-I_{a}\right]^{2}}{P-I_{a}+S_{\max }} \tag{4}
\end{equation*}
$$

- Another generalized approximation made on the basis of measuring storm runoff in small, agricultural watersheds under "normal conditions of antecedent wetness" is that

$$
I_{a} \approx 0.2 S_{\max }
$$

(5)

The few values actually tabulated in the 'original' report are 0.15-0.2 $S_{\text {max }}$.

- Thus

$$
\begin{equation*}
P-I_{a}=P-0.2 S_{\max } \tag{6}
\end{equation*}
$$

- Combination of these relations yields

$$
\begin{equation*}
R_{s}=\frac{\left[P-0.2 S_{\max }\right]^{2}}{\left[P+0.8 S_{\max }\right]} \tag{7}
\end{equation*}
$$

for all $P>I_{a}$. ELSE $R=0$.
Thus, the problem of predicting storm runoff depth is reduced to estimating a single value, the maximum retention capacity of the watershed, $S_{\text {max }}$.

The parameter $S_{\max }(\mathrm{mm})$ is related to a parameter called the Curve Number, which is an index of "storm-runoff generation capacity", varying from 0 to 100.


Rainfall. $P$ linchest

Relationship between rainfall and runoff (in depth) for a given event

- The entire rainfall-runoff response for various soilplant cover complexes is represented by the Curve Number (large oversemplification!).
- A higher curve indicates a large runoff response from a watershed with a fairly uniform soil with a low infiltration capacity.
- A lower curve is the smaller response expected from a watershed with a permeable soil, with a relatively high spatial variability in infiltration capacity.


## CNs are evaluated for many watersheds and related to:

- soil type (SCS soil types classified into Soil Hydrologic Groups on the basis of their measured or estimated infiltration behavior)
- vegetation cover and or land use practice
- antecedent soil-moisture content

| CLASSIFICATION | TYPE OF SOIL |
| :--- | :---: |
| A (low runoff potential) | Soils with high infiltration capacities, even when <br> thoroughly wetted. Chiefly sands and gravels, <br> deep and well drained. <br> Soils with moderate infiltration rates when <br> thoroughly wetted. Moderately deep to deep, <br> moderately well to well drained, with <br> moderately fine to moderately coarse textures. <br> S Soils with slow infiltration rates when thoroughly |
| wetted. Usually have a layer that impedes |  |
| vertical drainage, or have a moderately fine |  |
| to fine texture. |  |

Hydrologic Soil Groups are defined in SCS Soil Survey reports

## Classification of hydrologic properties of vegetation covers for estimating curve numbers (US Soil Conservation Service, 1972)

| VEGETATIVE COVER | HYDROLOGIC CONDITION |
| :--- | :--- |
| Crop rotation | Poor: Contain a high proportion of row crops, <br> small grains, and fallow. <br> Good: Contain a high proportion of alfalfa and <br> grasses. |
| Native pasture or range | Poor: Heavily grazed or having plant cover on <br> less than 50\% of the area. |
|  | Fair: Moderately grazed; 50-75\% plant cover. <br> Good: Lightly grazed; more than 75\% plant cover. <br> Permanent Meadow: 100\% grass cover. |
| Woodlands | Poor: Heavily grazed or regularly burned so that |
| litter, small trees, and brush are destroyed. |  |

Table 10-3 Runoff curve numbers for hydrologic soil-cover complexes under average conditions of antecedent moisture. (From U.S. Soil Conservation Service 1972.)

## Runoff Curve Numbers for hydrologic soilcover complexes under average antecedent moisture conditions

| IAND USI: OR COVER | TREAIMENI OR PRACTICE | HYDROIOGIK (ONDITION | IIYDROMOGAK <br> soll (iRelp |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | A | B | c | 1) |
| Fallow | Straight row | Poor | 77 | 86 | 91 | 94 |
| Row crops | Straight row | Poor | 72 | 81 | 88 | 91 |
|  | Straight row | Good | 67 | 78 | 85 | 89 |
|  | Contoured | Poor | 70 | 79 | 84 | 88 |
|  | Contoured | Good | 65 | 75 | 82 | 86 |
|  | Contoured and terraced | Poor | 66 | 74 | 80 | 82 |
|  | Contoured and terraced | Good | 62 | 71 | 78 | 81 |
| Small grain | Straight row | Poor | 65 | 76 | 84 | 88 |
|  | Straight row | Good | 63 | 75 | 83 | 87 |
|  | Contoured | Poor | 63 | 74 | 82 | 85 |
|  | Contoured | Good | 61 | 73 | 81 | 84 |
|  | Contoured and terraced | Poor | 61 | 72 | 79 | 82 |
|  | Contoured and terraced | Good | 59 | 70 | 78 | 81 |
| Close-sceded legumes or rotation meadow | Straight row | Poor | 66 | 77 | 85 | 89 |
|  | Straight row | Good | 58 | 72 | 81 | 85 |
|  | Contoured | Poor | 64 | 75 | 83 | 85 |
|  | Contoured | Good | 55 | 69 | 78 | 83 |
|  | Contoured and terraced | Poor | 63 | 73 | 80 | 83 |
|  | Contoured and terraced | Good | 51 | 67 | 76 | 80 |
| Pasture or range |  | Poor | 68 | 79 | 86 | 89 |
|  |  | Fair | 49 | 69 | 79 | 84 |
|  |  | Good | 39 | 61 | 74 | 80 |
|  | Contoured | Poor | 47 | 67 | 81 | 88 |
|  | Contoured | Fair | 25 | 59 | 75 | 83 |
|  | Contoured | Giood | 6 | 35 | 70 | 79 |
| Meadow (permanent) |  | Good | 30 | 58 | 71 | 78 |
| Woodlands (farm woodlots) |  | Poor | 45 | 66 | 77 | 83 |
|  |  | Fair | 36 | 60 | 73 | 79 |
|  |  | Good | 25 | 55 | 70 | 77 |
| Farmsteads |  |  | 59 | 74 | 82 | 86 |
| Roads. dirt |  |  | 72 | 82 | 87 | 89 |
| Roads. hard-surface |  |  | 74 | 84 | 90 | 92 |

# Curve Numbers for urban/suburban land covers (US Soil Conservation Service , 1975) 

| LAND USE | HYDROLOGIC SOIL GROUP |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | A | B | c | D |
| Open spaces, lawns, parks, golf courses, cemeteries, etc. |  |  |  |  |
| fair condition: grass cover on $50 \%$ to $75 \%$ of the area | 49 | 69 | 79 | 84 |
| Commercial and business area (85\% impervious) | 89 | 92 | 94 | 95 |
| Industrial districts ( $72 \%$ impervious) | 81 | 88 | 91 | 93 |
| Residential* |  |  |  |  |
| Average lot size Average \% Impervious ${ }^{\dagger}$ |  |  |  |  |
| $1 / 8$ acre or less 65 | 77 | 85 | 90 | 92 |
| $1 / 4$ acre 38 | 61 | 75 | 83 | 87 |
| $1 / 3$ acre 30 | 57 | 72 | 81 | 86 |
| $1 / 2$ acre 25 | 54 | 70 | 80 | 85 |
| 1 acre 20 | 51 | 68 | 79 | 84 |
| Paved parking lots, roofs, driveways, etc. ${ }^{\ddagger}$ | 98 | 98 | 98 | 98 |
| Streets and roads |  |  |  |  |
| Paved with curbs and storm sewers ${ }^{\ddagger}$ | 98 | 98 | 98 | 98 |
| Gravel | 76 | 85 | 89 | 91 |
| Dirt | 72 | 82 | 87 | 89 |

[^0]
## Accounting for the Antecedent Moisture Condition

## Table 1

| AMC <br> category | Rainfall depth in the previous 5 <br> days (mm) |  |
| :---: | :---: | :---: |
|  | Dormant <br> sesaon | Growing <br> season |
| AMC-I | $<12.7$ | $<35.6$ |
| AMC-II | $12.7-27.9$ | $35.6-53.3$ |
| AMC-III | $>27.9$ | $>53.3$ |

The previous table permit the computation of a CN which is valid for an average AMC (Antecedent Moisture Condition). It is possible to adapt the method to varying AMC based on the AMC category defined in Table 1, and then computing the corresponding CN values based on the relationships for CN(I) (valid for AMC-I) and $C N($ III) (valid for AMC-III).

## Example

It is required to compute the runoff depth (in mm and in cubic meters) for a $20 \mathrm{~km}^{2}$ catchment and for a storm event given by the following hyetograph (the hyetograph is the record of rainfall with time for a given storm):
hour 1: 20.0 mm
hour 2: 35.0 mm
hour 3: 15.0 mm
Use the CN method (curve number), taking a value of 60 for the $C N$.

$$
\begin{aligned}
& S=254\left(\frac{100}{60}-1\right)=169.3 \mathrm{~mm} \\
& I_{a}=0.2 S=33.9 \mathrm{~mm} \\
& R_{S}=\frac{(70-33.9)^{2}}{70+135.4} \cong 6 \mathrm{~mm} \\
& \text { which means } \\
& 6 \times 10^{-3} \times 20 \times 10^{6}=120 \times 10^{3} \mathrm{~m}^{3}
\end{aligned}
$$

## Esercise (2)

The SCS method allows one also to compute the runoff at each time step (i.e., not only at the event time scale).
To this end, the SCS relationship is applied at each time step in terms of cumulative quantities, and then subtracting at each time step the cumulative quantities computed at the previous time step.

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& \text { i.e. } \\
& 6 \times 10^{-3} \times 20 \times 10^{6}=120 \times 10^{3} \mathrm{~m}^{3}
\end{aligned}
$$

## Esercise (3)

Hour 1:
$P=20 \mathrm{~mm}<I_{a} \quad$ (no runoff)
Hour 2 :
during hr $1+$ ht 2 we have $55 \mathrm{~mm}>I_{a}$
$R_{s}=\frac{(55-33.9)^{2}}{55+135.4} \cong 2.3 \mathrm{~mm}$
then $R_{s}(\mathrm{hr} 2)=2.3 \mathrm{~mm}$
Hour $3:$
during $\mathrm{hr} 1+h r 2+h r 3$ we have $70 \mathrm{~mm}>I_{a}$
$R_{s}=\frac{(70-33.9)^{2}}{70+135.4} \cong 6.0 \mathrm{~mm}$
then $R_{s}($ ora 3$)=(6.0-2.3) \mathrm{mm}=3.7 \mathrm{~mm}$

## Application

Examining the hydrologic consequences of an extreme storm event in Este (31.05.1995)
catchment:
Rainfall depth:

70 ha
199.8 mm
(in two events: $1^{\circ}$ : 58.2 mm ; $2^{\circ}: 141.6 \mathrm{~mm}$ )

CN(II) catchment: $\mathrm{CN}(\mathrm{I})$ : CN(III): for $1^{\circ}$ event: condition AMC I; for $2^{\circ}$ event AMC III

## $1^{\circ}$ event

$$
\begin{aligned}
& S=254\left(\frac{100}{58}-1\right)=183.9 \mathrm{~mm} \\
& \text { if } \quad c=0.2 \\
& \Rightarrow P_{e}=\frac{(58.2-0.2 \cdot 183.9)^{2}}{58.2+0.8 \cdot 183.9}=2.2 \mathrm{~mm} \\
& \text { runoff coeff } .=\frac{2.2}{58.2}=0.04
\end{aligned}
$$

$2^{\circ}$ event

$$
\begin{aligned}
& S=254\left(\frac{100}{86}-1\right)=41.3 \mathrm{~mm} \\
& \text { if } \quad c=0.2 \\
& \Rightarrow P_{e}=\frac{(141.6-0.2 \cdot 41.3)^{2}}{141.6+0.8 \cdot 41.3}=101.8 \mathrm{~mm} \\
& \text { runoff coeff. }=\frac{101.8}{141.6}=0.72
\end{aligned}
$$

## Esercise (4)

Four raingauges measures rainfall during one storm event.
The measured rainfall depths and the Thiessen factors (for a given catchmet) for each raigauge are as follows:

Raingauge 1: 35 mm Thiessen weight=0.2
Raingauge 2: 45 mm Thiessen weight=0.2
Raingauge 3: 85 mm Thiessen weight=0.2
Raingauge 4: 10 mm Thiessen weight $=0.4$
The catchment is characterised by a $C N=75$, Ia $=0.2 \mathrm{~S}$, and it is $50 \mathrm{~km}^{2}$ wide.
Compute the runoff depth, in mm and in $\mathrm{m}^{3}$.


[^0]:    *Curve numbers are computed assuming the runoff from the house and driveway is directed toward the street with a minimum of roof water directed to lawns where additional infiltration could occur.
    ${ }^{\dagger}$ The remaining pervious areas (lawn) are considered to be in good pasture condition for these curve numbers.
    ${ }^{\text {f }}$ In some warmer climates of the country a curve number of 95 may be used.

